



1 ORIGINAL RESEARCH PAPER



2 Behaviour of Hot Mix Asphalt Incorporating Untreated and Treated  
3 Waste Cooking Oil

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8 Abstract

**AQ1** The recyclability of waste cooking oil (WCO) as rejuvenator for aged asphalt mixture improved the serviceability of pavement itself. Currently, the researcher is exploring the new potential of WCO as modifier for binder modification. However, **AQ2** the issue of compatibility properties in the modification of binder with WCO arises since the poor mechanical performance of asphalt mixture is globally recorded thus reflected the weakness of adhesion bonding inside the pavement material. Basically, the superior mechanical performance of asphaltic concrete exhibited good adhesion bonding between binder-aggregates interaction in bituminous mixture. In fact, the potential of high adhesiveness binding properties is affected by the chemical theory which is mostly related to the polarity factor. Therefore, it is vital to conduct the chemical analysis and microstructure observation to obtain a comprehensive understanding of the polar group behaviour for the internal structure in pavement material that influencing the adhesion performance for the structural arrangement material in the mixture. Therefore, excellent adhesion is capable of improving mechanical performance of Hot Mix Asphalt (HMA). The identification of chemical composition for polarity group determination was identified by using Gas Chromatography-Mass Spectrometry (GC-MS). Meanwhile, the adhesiveness measurement between binder-aggregate interactions in the mixture was observed with Field Emission Scanning Electron Microscope (FESEM) which physically resulting in mechanical performance by conducting resilient modulus test, dynamic creep test and indirect tensile strength (ITS) test. Results showed that the incompatibility characteristic is revealed between untreated WCO and conventional binder (PEN 60/70) based on the identification of polar and non-polar compounds interaction. Thereby, exhibits the existence of gap and void structure arrangement in HMA through FESEM visualization hence affecting the poor mechanical strength of asphalt mixture.

26 **Keywords** Waste cooking oil · Polarity · Adhesion · Hot mix asphalt

1 Introduction 27

The generation of kitchen waste particularly in WCO obtained from frying activities has increased due to the demand of food stuff. However, the common habits of disposing WCO into the kitchen sink have engaged many drawbacks on environmental issues. According to Zahoor et al. [37], the improper way of WCO's disposal has triggered the contamination of water and soil, disrupt the ecosystem of aquatic life and stimulate the wastewater sewage treatment cost for maintenance due to clogging problem. Hence, waste recycling of WCO is viewed as an alternative that can be adapted to cater the pollution matter. Moreover, the good adhesion mechanism between the binder and aggregates particles in bituminous mixture was related with an improvement of mechanical performance for asphalt mixture

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[36]. The adhesion characteristic was affected by the internal microstructure component. Fundamentally, the chemical theory is identified as significant major factors which contributes to the adhesion rate potential. According to Jose et al. [20], the characterisation of the material properties at the smaller length scale influencing the behaviour of bitumen at macroscopic measurement based on the determination from the chemical analysis, in which it can be identified based on the polarity group. Bahia et al. [10] and Moraes et al. [25] stated that the determination of polarity is categorised as the chemical composition of the material that is required to be chemically investigated. This is also supported by Xu et al. [35], in which a comprehensive understanding on the chemistry of binder is important as the physical and mechanical properties of the binder were influenced by the chemical composition possessed in asphalt binder materials. To understand the fundamental mechanism of adhesion properties comprehensively, the polarity group is determined by referring to the presence of chemical composition inside the pavement material that attributed to the high strength of adhesion binding between binder-aggregate in asphalt mixture [26]. The chemical analysis was evaluated by performing Gas Chromatography-Mass Spectrometry (GC-MS) for polarity group identification and microstructure observation through Field Emission Scanning Electron Microscope (FESEM) for adhesion performance investigation. The effectiveness of adhesion binding between binder-aggregate in the mixture is proven by the high strength to resist permanent deformation [1] through resilient modulus test, dynamic creep test, and indirect tensile strength (ITS) test.

Several studies focused on the microstructure observation of waste cooking oil (WCO) in modified binders have been reviewed. An analysis on the existence of chemical composition concerning functional group for modified binder by incorporating biodiesel by-product derived from WCO as a modifier was carried out by Sun et al. [30] by using Fourier Transform Infrared (FTIR). Based on the FTIR observation, the similarity of elemental composition between both materials of control binder and biodiesel by-product was recorded which consisted of saturated hydrocarbon, amides and aromatic compounds. However, a noticeable difference in functional group was identified wherein the control binder contains sulfinyl compounds meanwhile lipid compounds has existed in biodiesel by-product. Basically, lipids can be categorized in fat group, which is non-polar while asphalt binder in nature is identified as polar group. According to Nurdin et al. [27], the chemical standpoint stated that the fat group has high solubility in the same fat source component. It is expected that the incompatibility issue will occur during the substitution of biodiesel by-product derived from waste cooking oil in binder, which in turn contributed to the detrimental effect on the rheological performance, thereby affecting to the poor mechanical performance of asphalt mixture.

The addition of an oil-based source for binder modification shows an improvement in thermal cracking resistance at low temperature as reported by numerous previous researchers. However, the rutting resistance performance showed an adverse effect, in which the deterioration at high-temperature performance was observed with the addition of WCO as a modifier in binder. It is evidenced with the previous findings by Wen et al. [33] and Maharaj et al. [24], which stated that the poor rutting resistance performance at high temperature was recorded when WCO was substituted for asphalt binder modification. Practically, the softening effect is exhibited on the modified binders with WCO and it is expected that the soft binder contributes to poor performance in permanent deformation resistance due to unable to withstand with high-temperature exposure. Despite the minimum addition of WCO in binder, the detrimental effect is noticeable in effecting the modified binders, which in turn reduces adhesive properties to aggregate and thus leads to the poor mechanical performance of asphalt mixture. There is no conclusion that can be drawn and the justification is presently not well documented in explaining the reason for this mechanism. The justification of the main factor that attributed to this issue where it is due to the incompatibility characteristic between the asphalt binder and oil based source, thereby inducing instability problem in the chemical reaction between oil in modified binders and asphalt mixture [12]. This can be depicted that there is a constraint factor that has refused the occurrence molecule interaction in making a strong chemical bonding in the modified binders with WCO. Due to the arising issue, the investigation on the compatibility properties between WCO and asphalt binder becomes a major concern in this study. Therefore, it is significant to perform chemical analysis by conducting GC-MS to discover whether untreated WCO and treated WCO are compatible with the binder for binder modification based on the polarity group determination. Thereby, observing how the compatibility properties effecting to the adhesion performance between binder-aggregates which influencing particle arrangement inside the mixture through FESEM image visualization. Thus, high adhesiveness property is proven based on the superior mechanical performance of HMA.

## 2 Materials and Experimental Works

### 2.1 Virgin Binder

In this study, PEN 60/70 asphalt binder was used as control asphalt binder. The binder source was supplied by Eksklusif Alfa Enterprise located in Selangor, Malaysia and met all the requirements specified by the Malaysian Public Works Department standard. The penetration and softening point for binder PEN 60/70 are recorded as 62 dmm and 50 °C

144 which indicates that the physical properties of control binder  
145 are acceptable and allowed to be used for control binder as  
146 it follow the specification.

## 147 2.2 Untreated WCO and Treated WCO

148 Basically, two types of modifier were added into control binder  
149 to produce modified binders which can be divided  
150 into untreated WCO and treated WCO. For untreated WCO,  
151 this modifier is recognized as fresh/raw WCO which was  
152 collected from food restaurants and mostly disposed of with-  
153 out undergoing any proper treatment [21]. The collection of  
154 three types of WCO samples were denoted as WCO 1, WCO  
155 2, and WCO 3. The three WCO types were represented as  
156 WCO 1 is four repeated time usage, followed by WCO 2 is  
157 three repeated time usage and WCO 3 is two repeated time  
158 usage. These WCO samples were differentiated based on  
159 the various frequent duration times WCO was used thereby  
160 exhibit different acid values as shown in Table 1. Based on  
161 the preliminary findings, the physical and rheological perfor-  
162 mance of modified binders with untreated WCO is affected  
163 by the different quality of acid value wherein the lowest acid  
164 value (WCO 3) achieved the most excellent performance,  
165 especially in regard to the binder rheological properties ( $G^*/$   
166  $\sin \delta$ ) in terms of rutting resistance. In contrast, the highest  
167 acid value, represented as WCO 1, has shown low binder  
168 performance. It can be summarized that the WCO possessed  
169 the lowest acid value exhibit good binder performance.  
170 Therefore, WCO 3, with the lowest acid value, was selected  
171 for further pre-treatment of WCO by conducting transesteri-  
172 fication process in producing treated WCO to determine any  
173 performance improvement when the existing acid value was  
174 minimised through the chemical process.

175 Meanwhile, treated WCO is derived from the untreated  
176 WCO wherein this modifier underwent chemical treatment  
177 through the transesterification process. This chemical pro-  
178 cess is conducted when the untreated WCO is chemically  
179 reacted with methanol (alcohol) in the presence of sodium  
180 hydroxide (NaOH) as base catalysts to produce treated WCO.  
181 Prior to the process, a total of 1 mL NaOH and 600 mL  
182 of methanol were weighted and mixed together. Thereaf-  
183 ter, this solution was added into 100 mL of untreated WCO  
184 in a beaker containing a magnetic stirring bar to ensure a

185 homogeneous solution was obtained during the reaction. The  
186 1:6 ratio (oil:methanol) was selected in the chemical pro-  
187 cess as the optimum ratio based on the source of the WCO  
188 which originated from the palm oil, the frequency times  
189 of the WCO used during the cooking process, and existing  
190 acid value quality. This factor influencing the selection of  
191 appropriate optimum ratio of oil:methanol. Besides, previ-  
192 ous researcher proved that the usage of methanol is effective  
193 with ratio 6 to reduce acid value for untreated WCO. The  
194 minimization of acid value through the transesterification  
195 process is expected can enhance the physical and rheologi-  
196 cal performance of modified binder with treated WCO. The  
197 solution was heated on the hot plate until reached a tempera-  
198 ture of 65 °C and the reaction was started for a duration time  
199 of 1 h. The reaction was completed when the separation of  
200 two major by-products was observed namely, treated WCO  
201 at the upper phase and glycerol in the lower phase.

202 Figure 1 showed the viscosity result at 135 °C on modi-  
203 fied binder containing different WCO quality. The viscosity  
204 values decreased as WCO content increased. A similar trend  
205 was recorded for all the different WCO qualities and the vis-  
206 cosity decrement was sequentially observed from low qual-  
207 ity (WCO 1) followed by WCO 2 and high quality of WCO  
208 (WCO 3). Modified binder incorporated with WCO is lower  
209 than control binder at 0.7 Pa.s due to the natural WCO char-  
210 acteristic which provided fluidity properties. Thus, induced  
211 the sliding effect of particles inside the modified binder. It  
212 can be seen that, WCO 1 sample displayed the highest vis-  
213 cosity result, which was 0.6 Pa.s for 3%, 0.5 Pa.s for 4% and  
214 0.4 Pa.s for 5% of WCO and followed by WCO 2 sample  
215 which was 0.5 Pa.s (3%), 0.4 Pa.s (4%) and 0.3 Pa.s (5%),  
216 accordingly. The modified binder containing the 3%, 4% and  
217 5% of WCO 3 sample recorded the lowest viscosity which  
218 was 0.3 Pa.s as compared to WCO 1 and WCO 2 samples.

219 It can be noticed that the highest different viscosity  
220 decrement was recorded by WCO 3, which indicated soft  
221 and less viscous characteristic in WCO 3 modified binder.  
222 Due to the softer binder, the lowest internal friction was  
223 produced, thereby decreased the flow resistance which  
224 affected to the lowest viscosity result. In economic aspect,

Table 1 Acidity value content in WCO

Sample	Volume of KOH (ml)	Acid value (ml/g)	Conversion to FFA (based on oleic acid %)
WCO 1	31.7	3.55	1.78
WCO 2	25.3	2.83	1.42
WCO 3	14.8	1.65	0.83

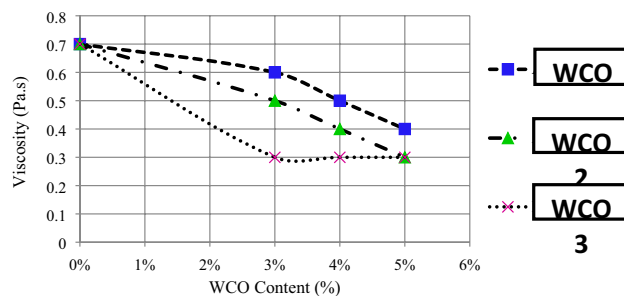


Fig. 1 Viscosity

low viscosity in binder is favourable due to low energy consumption with lower mixing, laying and compaction temperature thus save the construction cost (Sengoz & Isikyakar, 2008). The replacement of WCO in modified binder was beneficial for pavement construction by reducing the mixing and compaction temperature to minimise the operation cost.

The physiochemical properties of untreated and treated WCO are summarised in Table 2. Four main properties were examined, namely acid value, density, viscosity and water content. The transesterification process reduced the acid value from untreated WCO (1.65 ml/g) to treated WCO (0.54 ml/g). Meanwhile, treated WCO (0.81 kg/m<sup>3</sup>) had a lower density as compared to the untreated WCO which was 0.89 kg/m<sup>3</sup>. The density was predicted since the untreated WCO was thick relative to the treated WCO which was quite light in physical appearance. The viscosity result at temperature 135 °C for untreated WCO was significantly high (55.67 mpa.s) thus indicated the high internal friction. In contrast to treated WCO, which contained methyl ester, a lower viscosity was recorded as 1.15 mpa.s. Besides, the higher water content of treated WCO was observed as compared to untreated WCO. This is because the water was produced during the transesterification reaction. The noticeable difference between the two types of WCO was identified based on the color, in which untreated WCO was recognised as dark brown in colour while the treated WCO was light yellow.

**Table 2** Physiochemical properties of untreated and treated WCO

Properties	Untreated WCO	Treated WCO
Acid value (ml/g)	1.65	0.54
Density (kg/m <sup>3</sup> )	0.89	0.81
Dynamic viscosity (mpa.s)	55.67	1.15
Water content (ml)	0.085	6.46
Color	Dark brown	Light yellow

**Table 3** Summary of physical and rheological test for control and modified binders

Designation	WCO content (%)	Penetration (dmm)	Softening point (°C)	G*/sin δ (kPa)	Failure temperature (°C)
Control (60/70)	0%	68	49	0.78	70
Untreated WCO	5%	177	36	0.87	64
	10%	236	29	0.66	58
	15%	245	20	0.77	46
	20%	228	15	0.76	46
Treated WCO	5%	80	52	0.80	70
	10%	82	51	0.74	70
	15%	84	51	0.79	70
	20%	101	50	0.55	70

## 2.3 Hot Mix Asphalt (HMA) Incorporating with Untreated and Treated WCO in Modified Binders

Modified binders were prepared when the 5% of untreated WCO (at 5% optimum bitumen content) and treated WCO (at 4.9% optimum bitumen content) was added and mixed into the control binder (PEN 60/70). This percentage is selected based on the optimum value from physical test (penetration and softening point) and rheological test (Dynamic Shear Rheometer, DSR) among 0%, 5%, 10%, 15%, 20% of untreated and treated WCO, respectively as shown in Table 3. These materials were blended by using a high shear mixer at a constant speed of 1000 rpm for 1 h at 160 °C. Based on the physical result, the addition of higher percentage of untreated WCO content in binder increased the penetration value which indicates that the softer modified binders is produced thereby high tendency for rutting is expected. Meanwhile, the softening point is decreased when the untreated WCO is added which exhibit high-temperature susceptibility. The increasing trend of softening point was recorded for binder with treated WCO compared to untreated WCO contributed from similar polarity between binder and treated WCO attracted to one another forming a bonding connection which requires a higher temperature to break the carbon-hydrogen bond. Hence, explained the high softening point value for binder with treated WCO. The rheological findings show all the control and modified binder with untreated WCO sample achieved G\*/sin δ value is less than 1 kPa which does not meet AASTHO T315 requirement (G\*/sin δ should be ≥ 1 kPa). Basically, the value of G\*/sin δ which is less than 1 kPa denoted that the modified binders can withstand for rutting tendency at maximum temperature which represented by failure temperature. The highest failure temperature achieved by control sample (PEN 60/70) compared to the modified binder containing untreated WCO indicates the improvement of rutting resistance thereby proved that untreated WCO modified binders

290 prone to undergone deformation or rutting. This can be  
291 enhanced by using treated WCO in modified binder wherein  
292 the superior failure temperature is achieved comparable with  
293 control binder which is at 70 °C.

294 There were three different types of samples were tested  
295 for GC–MS in this research, wherein control binder, modi-  
296 fied binders with 5% untreated WCO and 5% treated WCO.  
297 Thereafter, control binder and modified binders with 5%  
298 untreated WCO and 5% treated WCO were used for asphalt  
299 mixture production. There were three types of asphalt mix-  
300 ture were prepared for FESEM observation in which control  
301 mixture, 5% untreated WCO mixture, and 5% treated  
302 WCO mixture. For asphalt mixture preparation, control  
303 binder, modified binders containing 5% untreated WCO  
304 and 5% treated WCO together with the aggregates was  
305 heated at 130 °C and 110 °C. Then, binder-aggregates were  
306 mixed at 170 °C until the aggregate was coated well with  
307 the binder. After the mixing process, the bituminous mix-  
308 ture was compacted by using a Marshall compactor for 75  
309 blow/face compaction standards for dense graded mixtures  
310 production. Based on the Marshall result, the highest Opti-  
311 mum Bitumen Content (OBC) value of 5.5% was achieved  
312 by control mixture, followed by the untreated WCO mixture  
313 with an OBC value of 5.0% and the lowest OBC value of  
314 4.9% was recorded by treated WCO mixture. Once the OBC  
315 obtained, the actual asphalt mixture with the exact OBC per-  
316 centages was prepared for further microstructure observation  
317 and mechanical testing for HMA containing untreated and  
318 treated WCO.

## 319 2.4 Experimental Methods

320 The experimental methods can be divided into three phases  
321 as presented in Fig. 2. In Phase 1, the polarity group classi-  
322 fication is identified to investigate the compatibility of WCO  
323 in binder. The compatibility characteristic is measured by  
324 using GC–MS. Then, the adhesion between binder-aggregate  
325 is visualized by conducting FESEM in Phase 2. For Phase 3,  
326 the HMA containing untreated and treated WCO is evaluated  
327 for strength measurement by performing mechanical test  
328 (resilient modulus, dynamic creep and ITS) thus eventually  
329 strengthen the fundamental findings on how the compatibil-  
330 ity of WCO in modified binders based on the polarity group  
331 identification affecting the adhesiveness properties between  
332 binder-aggregate in bituminous mixture.

## 333 2.5 Gas Chromatography–Mass Spectrometry (GC– 334 MS) Test

335 The types of GC–MS used in this study is GC–MS (QP2010  
336 Ultra Series) supplied from Shimadzu Malaysia Sdn. Bhd.  
337 GC–MS was performed to determine the quantitative analy-  
338 sis of chemical components in material for polarity group

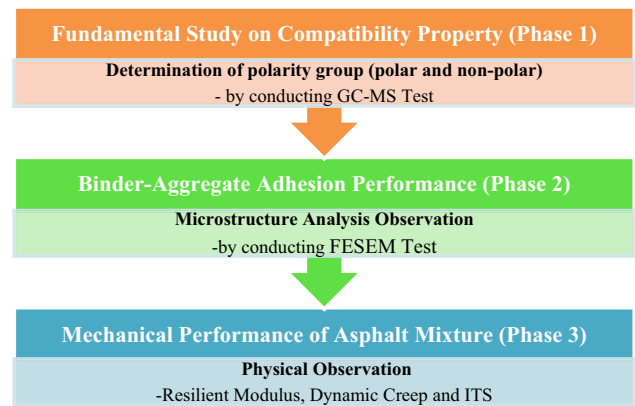


Fig. 2 Flowchart for Experimental Method (Phase 1, Phase 2, and Phase 3)

classification. This instrument comprises of gas chromatog- 339  
raphy (GC) coupled with mass spectrometry (MS) which 340  
enabling the separation, identification, and quantification 341  
of complex chemical compounds with low detection limit. 342

## 343 2.6 Field Emission Scanning Electron Microscope 344 (FESEM) Test

345 The visualization of the surface morphology of the mate- 345  
rial was observed by using FESEM. The FESEM instrument 346  
used in this test is from the Hitachi SU8020 model. The test 347  
was performed in accordance with ASTM E 2090 [3] pro- 348  
cedure. The material was sealed with tape on a metal stub 349  
during sample preparation. The prepared sample was coated 350  
with copper before putting into a gold coating machine. In 351  
obtaining a clear image, this material was vacuumed under 352  
the operation at 5.0 kV for 90 s. Then, the material was 353  
scanned up to 10,000× magnification and the visualization 354  
of morphological image structure was analyzed. 355

## 356 2.7 Resilient Modulus, Dynamic Creep, and ITS Test

357 In accordance with ASTM D7369-11, the elasticity behavior 357  
of the asphalt mixture after repetitive impact load was evalu- 358  
ated through the resilient modulus ( $M_R$ ) test. This test pro- 359  
vides an assessment of the resistance of the asphalt mixture 360  
to permanent deformation and its potential to return to its 361  
original condition after being subjected to 1000 kN loading. 362  
This test was carried out at 25 °C and 40 °C. The dynamic 363  
creep test was conducted by following the procedures out- 364  
lined in ASTM D3497-2003. This test is recognized as the 365  
simplest method for assessing the permanent deformation 366  
resistance during service life. The sample mixture was 367  
tested for 3600 cycles at 40 °C and 60 °C for about 30 min. 368  
The highest creep stiffness of the mixture indicates a high 369  
resistance to rutting potential. The ITS test was performed 370

371 in accordance with ASTM D6931-12. This test was con- 407  
 372 ducted to assess the performance of the bituminous mixtures 408  
 373 in terms of rutting and cracking potential. The ITS sample 409  
 374 was conditioned by placing it in an air bath at 25 °C for a 410  
 375 minimum of 4 h, prior to this test. 411

### 376 3 Results and Discussion 412

#### 377 3.1 Determination of Polarity Group from GC–MS 413 378 for Chemical Analysis 414

379 The molecule in the material can be classified into two dif- 415  
 380 ferent types of group namely, polar and non-polar. Uneven 416  
 381 distribution of electrons which enable the electrostatic inter- 417  
 382 action to occur is identified as a polar group in which it 418  
 383 consists of carboxyl, phosphate, amino, ketone, aldehyde, 419  
 384 and hydroxyl. Meanwhile for the non-polar group, as rec- 420  
 385 ognized from the methyl group, the charges have cancelled 421  
 386 out each other resulting no abundance of charges available 422  
 387 at the opposite side thus recognized as symmetrical elec- 423  
 388 tron distribution. In principle, the combination of the similar 424  
 389 group for instance polar-polar and non-polar-non-polar is 425  
 390 allowable to make good chemical bonding. Nevertheless, 426  
 391 the interaction between polar and non-polar group has no 427  
 392 capability to chemically react with each other as the exist- 428  
 393 ence of distinct group interaction. Numerous components 429  
 394 that existed in control binder are identified from carboxyl 430  
 395 group which represented by 1,2,4-benzenetricarboxylic acid 431  
 396 (-COOH-) as presented in Table 4. The presence of carbox- 432  
 397 ylic acid observed from GC–MS indicates that the chemical 433  
 398 composition was categorised as polar group. This is also 434  
 399 supported by Robertson [28], which stated that the detec- 435  
 400 tion of carboxylic acid in binder constituent was catego- 436  
 401 rized as a polar group and had high adhesiveness properties 437  
 402 towards the dry aggregates. Meanwhile, the highest compo- 438  
 403 nent presented in the modified binders containing untreated 439  
 404 WCO were recorded by the components of Cyclotrisiloxane, 440  
 405 hexamethyl (C<sub>6</sub>H<sub>18</sub>O<sub>3</sub>Si<sub>3</sub>) and Tetrasiloxane, decame- 441  
 406 thyl (C<sub>10</sub>H<sub>30</sub>O<sub>3</sub>Si<sub>4</sub>). The similarity presence of methyl (CH<sub>3</sub>) 442

between these two components was notable which indicates 407  
 that the modified binders incorporating untreated WCO was 408  
 categorised as a non-polar group. A noteworthy point that 409  
 wants to be emphasized that the modification of binder with 410  
 the addition of untreated WCO has given effect to the unsta- 411  
 ble and imbalance of chemical composition inside the mate- 412  
 rial. This is due to the no attraction between the different 413  
 polarity groups and refused to make chemical interaction. 414  
 Thereby, it is proved that poor compatibility between these 415  
 two materials as the non-equilibrium of chemical reaction 416  
 existed between binder and untreated WCO due to the dif- 417  
 ferent polarity issue. The formation of ketone group (C=O) 418  
 which represented as a polar group was mostly presented 419  
 in modified binders containing treated WCO by the detec- 420  
 tion from 2,4,6-Cycloheptatrien-1-one, 3,5-bis-trimethyl- 421  
 silyl- (C<sub>13</sub>H<sub>22</sub>OSi<sub>2</sub>) as similar with the chemical composi- 422  
 tion in the control binder. Overall, it can be summarized 423  
 that modified binders with treated WCO was classified as 424  
 a polar group. An equilibrium and stable chemical reaction 425  
 was produced between control binder and treated WCO due 426  
 to the similarity of polar group which induced an attrac- 427  
 tion between these materials. This indicated that the binder 428  
 and treated WCO are compatible with each other, thereby 429  
 improved the superior adhesiveness with the aggregate in 430  
 asphalt mixtures. 431

#### 432 3.2 Adhesion Between Binder-Aggregate 433 434 for Structural Arrangement in Mixture 434 from FESEM Microstructure Visualization 434

Figure 3 depicted the FESEM image for the control mix- 435  
 ture, 5% untreated WCO mixture and 5% treated WCO 436  
 mixture. Based on the microstructure image visualized by 437  
 FESEM, the presence of more voids at certain points was 438  
 observed for control mixture as compared to the treated 439  
 WCO mixture. It can be noticed that the structure of con- 440  
 trol mixture less compacted in comparison with the treated 441  
 WCO mixture due to the availability of voids in the bitu- 442  
 minous mixture. This can be related with the natural prop- 443  
 erty of the control binder which has high viscosity thus 444

**Table 4** Chemical composition for control and modified binder with untreated and treated WCO

Types	Control binder (PEN 60/70)	Modified binders with 5% untreated WCO	Modified binders with 5% treated WCO
Molecule	1,2,4 benzenetricarboxylic acid	Cyclotrisiloxane, hexamethyl- Tetrasiloxane, decamethyl-	2,4,6-Cycloheptatrien-1-one, 3,5-bis tri-methylsilyl-
Chemical Formula	-COOH-	CH <sub>3</sub>	C=O
Area (%)	0.4805	0.4718 0.3737	0.5465
Polarity Group	Polar	Non-polar	Polar

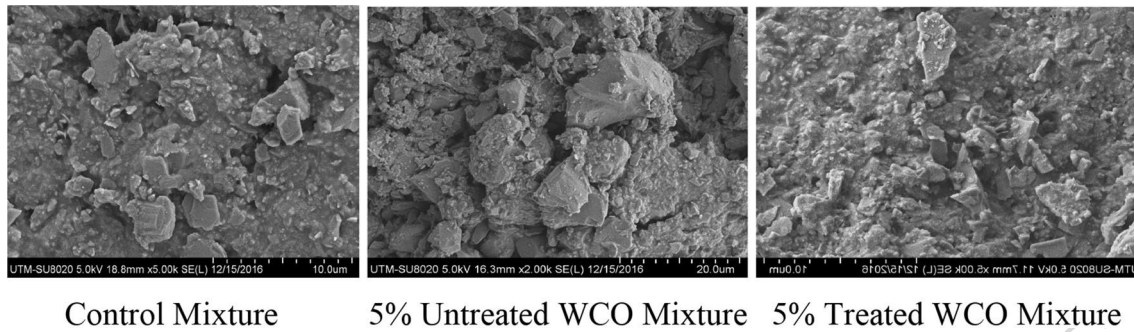


Fig. 3 Microstructure observation visualized from FESEM image

445 affected to the low flow performance [19]. This is also  
 446 supported by Hefer and Little [16], which stated that low  
 447 flow performance affected the low coatability or wettabil-  
 448 ity performance to coat the aggregates entirely thereby  
 449 reduced the adhesion strength between binder-aggregate  
 450 in the asphalt mixture. The incapability of control binder  
 451 to coat or cover all areas in the bituminous mixture due to  
 452 the low flow binder performance produced a non-uniform  
 453 coating process. Therefore, it can be emphasized that the  
 454 presence of voids in the control mixture was affected by  
 455 the improper coating process of binder for the aggregates  
 456 particles. The existence of voids caused the mixture to  
 457 be un-compacted hence reduced the stability performance  
 458 which affected the low mechanical strength relative to the  
 459 treated WCO mixture. On the contrary, the high wettability  
 460 potential of modified binders with treated WCO to coat the  
 461 aggregates indicates the enhancement of flow performance  
 462 due to the fluidity properties possessed by treated WCO.

463 For untreated WCO mixture, the existence of voids  
 464 were very noticeable as compared to control and treated  
 465 WCO mixtures. The increment of the gaps or voids exist-  
 466 ence in untreated WCO mixture was related to the reduc-  
 467 tion of the adhesion performance between the contacted  
 468 materials due to the expectation of different polarity group  
 469 interaction between modified binders with untreated WCO  
 470 and aggregate surface. This was agreed by Xiao et al. [34]  
 471 which stress out that the chemistry of the binder itself in  
 472 terms of polarity and constitution influenced the interac-  
 473 tion performance between binder-aggregate in the bitumi-  
 474 nous mixture. It can be proved that the structure arrange-  
 475 ment of the mixture was affected by the polarity factor as  
 476 the combination of polar and non-polar component has  
 477 caused the disintegration of the particle in the mixture  
 478 which leads to the de-bonding process between modified  
 479 binders with untreated WCO and aggregate particles. Due  
 480 to the incompatibility polarity group issue, it has effected  
 481 the poor mechanical performance of untreated WCO mix-  
 482 ture as well as compared to the other mixtures.

The surface microstructure for the treated WCO mixture  
 illustrated in FESEM image indicates an improvement of the  
 adhesion performance in the bituminous mixture whereby  
 the modified binders with treated WCO exhibited better  
 attraction to the polar surface aggregates. Another inter-  
 pretation indicated that the similarity of polarity group  
 contributes to the high affinity between polar modified binders  
 containing treated WCO and polar aggregates. No existence  
 of notable voids was observed in the bituminous mixture,  
 and therefore it can be noticed that the treated WCO mixture  
 was more compacted relative to the control and untreated  
 WCO mixtures. The visualization of the compacted mixture  
 from FESEM image proved that there was an improvement  
 in the adhesion properties between the aggregates particles  
 and modified binders with treated WCO.

### 3.3 Mechanism of Adhesion Between Bitumen-Aggregates Based on Microstructure Observation

The similar polarity groups induced the existence of a chem-  
 ical interaction to occur wherein the polar reacted with other  
 polar groups and vice versa. In contrast, the chemical reac-  
 tion was not observed as no attraction existed between dif-  
 ferent polarity groups due to the internal molecule repulsion  
 with each other to avoid chemical bonding interaction. The  
 GC-MS result from previous analysis summarised that the  
 control binder and treated WCO was categorised as polar  
 group while on the contrary, non-polar group represented by  
 untreated WCO as it exhibited hydrophobic behaviour [7].  
 This is also supported by Freemantle [14], which claimed  
 that as the binder possess alcohol, amine, carboxyl, phenolic,  
 and thiol, thereby it can be considered as a polar compound.  
 According to Nurdin et al. [27], non-polar is recognised as  
 hydrophobic (oil/fat loving) which is has a high solubility  
 in fat source component only. In contrast, the polar group  
 can be described as hydrophilic (water-loving) which can  
 be dissolved in water. This theory is related to the common

519 phenomena in which it can be observed that the oil (non-  
520 polar) based cannot be dissolved in water (polar) as the sepa-  
521 ration between oil at the upper layer and water at the lower  
522 layer is exhibited.

523 The modification of binder with untreated WCO had  
524 raised some polarity characteristic issues wherein the dif-  
525 ferent polarity possessed by the control binder and untreated  
526 WCO caused the repulsion between the particles inside the  
527 material. It indicates that the refusal interaction occurs to  
528 form good bonding as the particle pushed between each  
529 other thereby caused the rupture, deterioration and weaken  
530 the chemical bond in modified binders thus lead to the  
531 decrement of cohesive strength in the materials. It has to  
532 be emphasized that the compatibility properties between  
533 binder and untreated WCO were reduced due to the interac-  
534 tion of different polarity group which resulted in adverse  
535 rheological performance as evidenced by the poor rutting  
536 resistance with lowest failure temperature [9]. Testing for  
537 universal sorption device (USD) conducted by Guarin et al.  
538 [15] discovered that the stone surface was categorised as  
539 polar which had a high affinity towards polar liquid. This is  
540 supported by Volpe and Siboni [32] and Bhasin and Little  
541 [11] which confirmed that the aggregates surface derived  
542 from polar group naturally. An attraction between molecules  
543 in the modified binders containing treated WCO and the  
544 polar surface of aggregates was existed due to the similar-  
545 ity of polar groups thereby increased the adhesion perfor-  
546 mance of the binder-aggregates in the bituminous mixture  
547 [14]. However, the opposite polarity group possessed by the  
548 modified binders with untreated WCO, being a non-polar  
549 group, reduced adhesion to the polar surface of aggregates  
550 because no attraction bonds existed owing to the different  
551 polarity groups thus led to debonding. It can be summarized  
552 that the polar attraction between molecules in the asphalt  
553 binder and polar surface of the aggregates influenced an  
554 improvement of superior adhesion performance between  
555 binder-aggregates interactions. In contrast, a notable dif-  
556 ferent polarity group has worsened the adhesion binding  
557 performance of modified binders with untreated WCO for  
558 coating the aggregates.

### 559 3.4 Mechanical Performance Evaluation Through 560 Resilient Modulus, Dynamic Creep and ITS 561 for HMA

562 The summarised results for the resilient modulus of control,  
563 5% untreated WCO and 5% treated WCO mixtures  
564 which were tested at 25 °C and 40 °C is illustrated in Fig. 4.  
565 Based on the result, an increment of  $M_R$  was achieved from  
566 2681 MPa for untreated WCO mixture to 5977 MPa for  
567 treated WCO mixture in which it indicated that the chemi-  
568 cal modification enhanced the resilient modulus perfor-  
569 mance. Surprisingly, the obtained results showed that the

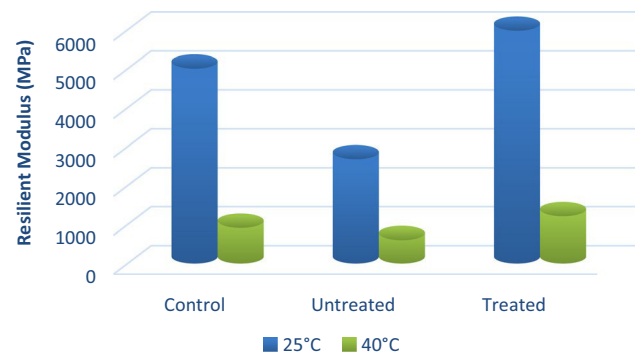


Fig. 4 Resilient modulus ( $M_R$ ) for control and modified mixture containing untreated and treated WCO

Table 5 The independent samples t-test for resilient modulus

Independent samples test		Resilient modulus		
		Equal variances assumed	Equal variances not assumed	
Levene's test for equality of variances	<i>F</i>	23.386		
	Sig.	0.000		
<i>t</i> -test for equality of means	<i>t</i>	7.333	7.333	
	<i>df</i>	16	8.523	
	Sig. (2-tailed)	0.000	0.000	
	Mean difference	3644.333	3644.333	
	Std. error difference	496.947	496.947	
	95% confidence interval of the difference			
		Lower	2590.853	2510.506
	Upper	4697.814	4778.160	

570 mixes containing treated WCO exhibited superior results to  
571 the control mix sample by exceeding the resilient modulus  
572 for the control mixture at 5006 MPa. Besides, the resilient  
573 modulus was reduced at 40 °C temperature as compared to  
574 25 °C, which was recorded as 601 MPa for untreated WCO  
575 mixture, 915 MPa for control mixture and 1214 MPa for  
576 treated WCO mixture. This indicated, as expected, that the  
577 treated WCO mixture achieved the highest resilient mod-  
578 ulus when compared to the other mixtures. According to  
579 this trend, it shows that treated WCO mixture exhibited the  
580 highest achievement of resilient modulus, while the lowest  
581 resilient modulus was recorded by untreated WCO mixture  
582 at both temperatures.

583 An independent *t*-test analysis was performed to com-  
584 pare the resilient modulus performance between control,  
585 untreated WCO and treated WCO mixtures at two differ-  
586 ent temperatures of 25 °C and 40 °C, respectively as shown  
587 in Table 5. The Levene's test for equality of variances  
588 showed that the population variance was equal based on



589 the Sig. value of 0.000. Besides, the t-test for equality of  
590 means recorded the 2-tailed value for 0.000, which is less  
591 than 0.05. Since  $p < 0.05$ , therefore the null hypothesis was  
592 rejected. Another interpretation indicated that a difference of  
593 resilient modulus results between control, untreated WCO,  
594 and treated WCO, at both temperatures, was statistically  
595 significant.

596 Figure 5 presents dynamic creep stiffness results which  
597 conducted at both temperature of 40 °C and 60 °C. The  
598 increment of creep stiffness at 40 °C was recorded from  
599 untreated WCO mixture (196.15 MPa), control mixture  
600 (264.29 MPa) and treated WCO mixture (330.40 MPa). This  
601 implied that the treated WCO mixture exhibited the highest  
602 achievement of creep stiffness as compared to the control  
603 and untreated WCO mixtures. In addition, it proved that the  
604 rutting resistance was improved in treated WCO mixture as  
605 evidenced by the increased of creep stiffness strength. The  
606 improvement of creep stiffness in treated WCO mixture indicated  
607 a high rutting resistance, thereby reduced the tendency  
608 of permanent deformation to occur. Besides, the decrement  
609 of creep stiffness was noticed at 60 °C temperature as compared  
610 to 40 °C. However, the result reported a conflicting  
611 finding with temperature 40 °C, in which the control mixture  
612 achieved the highest creep stiffness for 85.96 MPa at  
613 60 °C. In contrast, the creep stiffness result was recorded  
614 slightly lower for treated WCO (16.04 MPa) and untreated  
615 WCO mixtures (11.33 MPa) in comparison with the control  
616 mixture.

617 The data was further extensively analysed by an independent  
618 t-test analysis to compare the creep stiffness performance for  
619 control, untreated WCO and treated WCO mixture samples for  
620 different test temperatures at 40 °C and 60 °C, correspondingly  
621 as presented in Table 6. The null hypothesis stated that the mean  
622 creep stiffness for three different types of mixture at low  
623 temperature (40 °C) and high temperature (60 °C) was equal  
624 ( $H_0: \mu_{40^\circ\text{C}} = \mu_{60^\circ\text{C}}$ ). The 2-tailed value was recorded  
625 as 0.000 which is less than 0.05, hence the

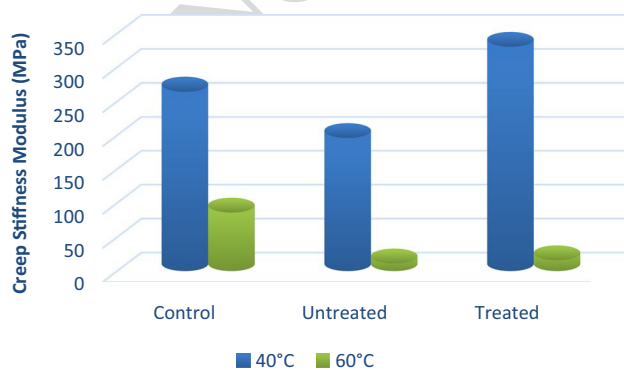


Fig. 5 Dynamic creep stiffness for control and modified mixture containing untreated and treated WCO

Table 6 The independent samples *t*-test for creep stiffness

Independent samples test		Creep stiffness	
		Equal variances assumed	Equal variances not assumed
Levene's test for equality of variances	<i>F</i>	1.188	
	Sig.	0.292	
<i>t</i> -test for equality of means	<i>t</i>	9.894	9.894
	<i>df</i>	16	13.392
	Sig. (2-tailed)	0.000	0.000
	Mean difference	225.84889	225.84889
	Std. error difference	22.82732	22.82732
	95% confidence interval of the difference		
	Lower	177.45712	176.67988
	Upper	274.24066	275.01790

626 null hypothesis was rejected. This implied that the mean  
627 difference of creep stiffness for control, untreated WCO and  
628 treated WCO mixtures at both temperatures of 40 °C and  
629 60 °C was statistically significant.

630 The ITS which tested at 25 °C is demonstrated in Fig. 6.  
631 ITS result shows that untreated WCO mixture was represented  
632 as 678 kPa, which was recorded lower as compared to the  
633 control mixture (1211 kPa) and treated WCO mixture  
634 (1349 kPa). This implied that the treated WCO mixture  
635 exhibited the highest tensile strength while the lowest tensile  
636 strength was recorded by untreated WCO mixture. This  
637 attraction increased the adhesion of the modified binder with  
638 treated WCO to the aggregates and led to the good bonding  
639 between the materials. In the bituminous mixture, the contact  
640 point between the aggregate particles was observed when the  
641 bitumen held the aggregate skeleton in position. This result  
642 implied that the presence of the binder produced an efficient  
643 aggregate binding that reduced the tendency of flow movement  
644 of the bituminous composition in the mixture, thereby increasing  
645 the strength performance of the

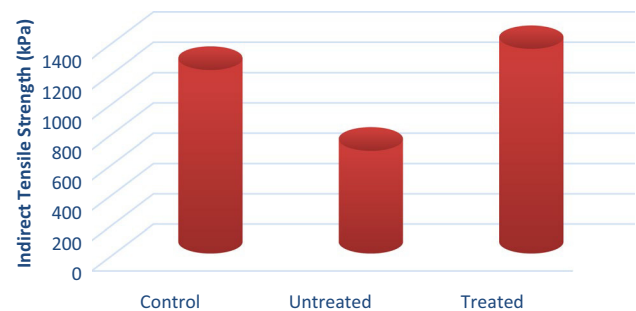


Fig. 6 ITS result for control and modified mixture containing untreated and treated WCO

**Table 7** One way ANOVA for ITS

Source	Sum of squares	df	Mean square	F	p
Between groups	753,374.000	2	376,687.000	1855.601	0.000
Within groups	1218.000	6	203.000		
Total	754,592.000	8			

646 treated WCO mixture. In summary, the better performance  
647 of tensile strength was recorded by treated WCO mixture as  
648 compared to the control and untreated WCO mixtures, which  
649 directly proved that the chemical treatment had enhanced the  
650 treated WCO performance in asphalt mixture.

651 A statistical analysis by using SPSS software was con-  
652 ducted for control and modified mixtures incorporating  
653 untreated and treated WCO as shown in Table 7. The data  
654 were analysed based on the 95% confident interval ( $\alpha=0.05$ ).  
655 The one-way independent analysis of variance (ANOVA)  
656 was performed to evaluate the significance of different  
657 asphalt mixture samples, namely control, untreated WCO  
658 and treated WCO mixtures on the indirect tensile strength  
659 (ITS) performance. The null hypothesis for analysis was that  
660 the mean of different mixture samples for ITS was equal  
661 ( $H_0: \mu_{\text{control mixture}} = \mu_{\text{untreated wco mixture}} = \mu_{\text{treated wco mixture}}$ ).  
662 The ANOVA result displayed implied that there was a sta-  
663 tistically significant difference in indirect tensile strength  
664 exhibited by the different asphalt mixture which consisted of  
665 control, untreated WCO and treated WCO mixtures as evi-  
666 denced by the  $p$  value as 0.000. Hence, the null hypothesis  
667 was rejected since the  $p$ -value was less than 0.05 and indi-  
668 cated that the ITS performance was influenced by the dif-  
669 ferent types of mixes namely control, untreated and treated  
670 asphalt mixture.

## 671 4 Conclusion

672 The GC–MS result discovered the identification of polar  
673 compounds in control binder and treated WCO while  
674 untreated WCO was recognised as a non-polar compound  
675 according to the chemical composition analysis. Based on  
676 the determination of polarity group, an incompatibility issue  
677 was identified and raised between binder and untreated  
678 WCO thereby justified how this polarity group contribute  
679 to the low adhesion bonding performance between modi-  
680 fied binders with untreated WCO and aggregate. The decre-  
681 ment of adhesion properties was affected by the different  
682 polarity group between aggregate and modified binders  
683 with untreated WCO thus lead to the adverse mechanical  
684 performance in the untreated WCO mixture. Meanwhile,  
685 the FESEM image illustrated that non-existence of voids is  
686 observed for treated WCO mixture thereby exhibiting more

compacted arrangement as compared to the control and  
untreated WCO mixtures. It can be noticed that, an improve-  
ment of the adhesion performance between the modified  
binders containing treated WCO and aggregate is attained as  
evidenced by the compact structure arrangement in bitumi-  
nous mixture. Meanwhile, the presence of voids were notice-  
able in untreated WCO mixture which attributed to the inter-  
action of different polarity group between untreated WCO  
(non-polar) and polar surface aggregate. The reduction of  
adhesion bonding performance between the contacted mate-  
rials was influenced by the interaction of opposite polarity  
group thus produced the notable gap/voids in the untreated  
WCO mixture. The relationship between fundamental theory  
on compatibility property based on the polarity group and  
adhesion of binder-aggregate is proved from mechanical  
test in which superior performance was recorded by treated  
WCO mixture compared to control mixture and untreated  
WCO mixture based on the highest strength obtained for  
resilient modulus, creep stiffness and ITS.

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## References

1. Alnadish, A. M., Aman, M. Y., Katman, H. Y. B., & Rasdan, M. (2021). Laboratory assessment of the performance and elastic behavior of asphalt mixtures containing steel slag aggregate and synthetic fibers. *International Journal of Pavement Research and Technology*, 14, 473–481
2. American Society for Testing Materials. (2000). *ASTM D907–96a. Standard terminology of adhesives. Annual book of ASTM Standards*. ASTM International.
3. American Society for Testing Materials. (2012). *ASTM E 2090. Standard specification test method for optical and scanning electron microscopy*. ASTM International.
4. American Society for Testing Materials. (2003). *ASTM D3497. Standard test method for dynamic modulus of asphalt mixtures*. ASTM International.
5. American Society for Testing Materials. *ASTM D6931-12, Standard test method for indirect tensile (IDT) strength of bituminous mixtures*. ASTM International.
6. American Society for Testing Materials. *ASTM D7369-11, Standard test method for determining the resilient modulus of bituminous mixtures by indirect tension test*. ASTM International.
7. Andrade, I. C., Santiago, J. P., Sodre, J. R., Pathiyamattom, J. S., & Guerrero-Fajardo, C. A. (2014). Transesterification reaction of waste cooking oil and chicken fat by homogeneous catalysis. *Journal of Chemistry and Chemical Engineering*, 8, 736–743
8. Azahar, W. N. A. W., Jaya, R. P., Hainin, M. R., Bujang, M., & Ngadi, N. (2017). Mechanical performance of asphaltic concrete incorporating untreated and treated waste cooking oil. *Construction and Building Materials*, 150, 653–663
9. Azahar, W. N. A. W., Jaya, R. P., Hainin, M. R., Bujang, M., & Ngadi, N. (2016). Chemical modification of waste cooking oil to improve the physical and rheological properties of asphalt binder. *Construction and Building Materials*, 126, 218–226
10. Bahia, H. U., Hanz, A., Kanitpong, K., & Wen, H. (2007). *Testing method to determine aggregate/asphalt adhesion properties and*

- 743 potential moisture damage WHRP 07–02. Wisconsin Highway  
744 Research Program.
- 745 11. Bhasin, A., & Little, D. N. (2007). Characterization of aggregate  
746 surface energy using the universal sorption device. *Journal of*  
747 *Material Civil Engineering*, 19, 634–641
- 748 12. Borhan, M. N., Suja, F., Ismail, A., & Rahmat, R. A. O. K. (2009).  
749 The effects of used cylinder oil on asphalt mixes. *European Jour-*  
750 *nal of Scientific Research*, 28(3), 398–411
- 751 13. De Oliviera, R. R. L., Albuquerque, D. A. C., Cruz, T. G.  
752 S., Yamaji, F. M., & Leite, F. L. (2012). Measurement of the  
753 Nanoscale Roughness by atomic force microscopy: basic princi-  
754 ples and applications. In V. Bellitto (Ed.), *Atomic force micros-*  
755 *copy-imaging, measuring and manipulating surfaces at the*  
756 *atomic scale*. (pp. 147–174). Intech. ISBN: 978-953-51-0414-8.  
757 (Chapter 7).
- 758 14. Freemantle, M. (1999). Asphalt. *Chemical and Engineering News*,  
759 77(47), 81
- 760 15. Guarin, A., Khan, A., Butt, A. A., Birgisson, B., & Kringos, N.  
761 (2016). An extensive laboratory investigation of the use of bio-oil  
762 modified bitumen in road construction. *Construction and Building*  
763 *Materials*, 106, 133–139
- 764 16. Hefer, A., & Little, D. (2005). *Adhesion in bitumen-aggregates*  
765 *system and quantification of the effect of water on the adhesive*  
766 *bond*. Research Report ICAR/505-1. (p. 22). Texas Transportation  
767 Institute.
- 768 17. Jabatan Kerja Raya Malaysia (JKR) (2008). *Standard specification*  
769 *for road works, Section 4: Flexible Pavement*. No. JKR/SPI/2008-  
770 S4, pp. S4-58-S4-69.
- 771 18. Jager, A., Lackner, R., Eisenmenger-Sitter, C., & Blab, R. (2004).  
772 Identification of four materials phases in bitumen by atomic force  
773 microscopy. *Roads Materials and Pavement Design*, 5, 9–24
- 774 19. Jeffry, S. N. A., Jaya, R. P., Abdul Hassan, N., Yaacob, H., &  
775 Satar, M. K. I. M. (2018). Mechanical performance of asphalt  
776 mixture containing nano-charcoal coconut shell ash. *Construction*  
777 *and Building Materials*, 173, 40–48
- 778 20. Jose, P. A. M., Jorge, S. D., Vivian, B. M., Ellen, R. C., Fabricio,  
779 L. V., & Luis, L. S. (2015). Morphological analysis of bitumen  
780 phases using atomic force microscopy. *Roads Materials and Pave-*  
781 *ment Design*, 16(1), 138–152
- 782 21. Kheang, L. S., May, C. Y., Foon, C. S., & Ngan, M. A. (2006).  
783 Recovery and conversion of palm olein-derived used frying oil  
784 to methyl esters for biodiesel. *Journal of Oil Palm Research*, 18,  
785 247–252
- 786 22. Lesueur, D. (2009). The colloidal structure of bitumen: conse-  
787 quences on the rheology and on the mechanisms of bitumen modi-  
788 fication. *Advance In Colloidal and Interface Science*, 145(1–2),  
789 42–82
- 790 23. Loeber, L., Sutton, O., Morel, J., Valleton, J. M., & Muller, G.  
791 (1996). New direct observation of asphalt and asphalt binder by  
792 scanning electron microscopy and atomic force microscopy. *Jour-*  
793 *nal of Microscopy*, 182(1), 32–39
- 794 24. Maharaj, R., Harry, V. R., & Mohamed, N. (2015). Rutting and  
795 fatigue cracking resistance of waste cooking oil modified trinidad  
796 asphaltic materials. *The Scientific World Journal*. <https://doi.org/10.1155/2015/385013>
- 797 25. Moraes, R., Velasquez, R. and Bahia, H. (2011). Measuring effect  
798 of moisture on asphalt-aggregate bond with the bitumen bond  
799 strength test. *Transportation Research Board Annual Meeting*,  
800 Washington, D.C.
- 801 26. Mugume, R. B., & Musumba, S. (2021). Contribution of reactive  
802 aggregates towards the performance of in-service asphalt pave-  
803 ments. *International Journal of Pavement Research and Technol-*  
804 *ogy*, 14, 530–536
- 805 27. Nurdin, S., Yunus, R. M., Nour, A. H., Gimbut, J., Azman, N. A.  
806 N., & Sivaguru, M. V. (2016). Restoration of waste cooking oil  
807 (WCO) using alkaline hydrolysis technique (ALHYT) for future  
808 biodetergent. *ARKN Journal of Engineering and Applied Science*,  
809 11(10), 6405–6410
- 810 28. Robertson, R. E. (2000). Chemical properties of asphalt and their  
811 effects on pavement performance. In *Transportation Research*  
812 *Board*, No. 499, Washington, D.C.
- 813 29. Singh, M., Kumar, P., & Maurya, M. R. (2013). Strength charac-  
814 teristics of SBS modified asphalt mixes with various aggregates.  
815 *Construction and Building Materials*, 41, 815–823
- 816 30. Sun, Z., Yi, J., Huang, Y., Feng, D., & Guo, C. (2015). Investiga-  
817 tion of the potential application of biodiesel by-product as asphalt  
818 modifier. *Road Materials and Pavement Design*, 17(3), 737–752
- 819 31. Tarrar, A. R., & Wagh, V. P. (1992). *The effect of the physical*  
820 *and chemical characteristics of the aggregate on bonding*. Report  
821 SHRP-A/UIR-91–507. Strategic Highway Research Program  
822 National Research Council.
- 823 32. Volpe, C. D., & Siboni, S. (2000). Acid base surface free ener-  
824 gies of solids and the definition of scales in the Good Van Oss  
825 Chaudhury Theory. *Journal of Adhesive Science Technology*,  
826 14(2), 235–272
- 827 33. Wen, H., Bhusal, S., & Wen, B. (2013). Laboratory evaluation of  
828 waste cooking oil-based bioasphalt as an alternative binder for hot  
829 mix asphalt. *Journal of Materials in Civil Engineering*, 25(10),  
830 1432–1437
- 831 34. Xiao, F. P., Amirhanian, S. N., & Juang, C. H. (2007). Rutting  
832 resistance of rubberized asphalt concrete pavements containing  
833 reclaimed asphalt pavement mixtures. *Journal of Materials in*  
834 *Civil Engineering*, 19, 475–483
- 835 35. Xu, M., Yi, J., Feng, D., Huang, Y., & Wang, D. (2016). Analysis  
836 of adhesive characteristic of asphalt based on atomic force micros-  
837 copy and molecular dynamics simulation. *ACS Applied Materials*  
838 *and Interfaces*, 8(19), 12393–12400
- 839 36. Yan, Z., Liu, W., Chen, J., & Dongzhao, J. (2021). Pavement con-  
840 ductive wearing surface with graphite heating film de-icing poten-  
841 tial and performance experimental study. *International Journal of*  
842 *Pavement Research and Technology*, 14, 688–696
- 843 37. Zahoor, M., Nizamuddin, S., Madapusi, S., and Giustozzi, F.  
844 (2020). Sustainable asphalt rejuvenation using waste cooking oil:  
845 a comprehensive review. *Journal of Cleaner Production*, 123304.
- 846